California Bearing Ratio (CBR)

California Bearing Ratio (CBR) is a penetration test for evaluation of the mechanical strength of road subgrades, sub-base and base courses. It was developed by the California Department of Transportation.

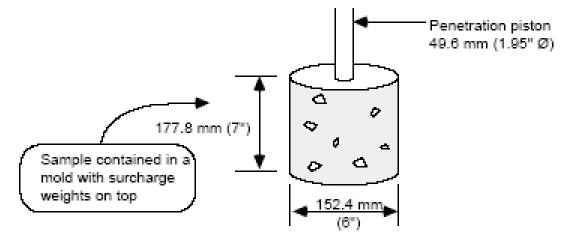
The test is performed by measuring the pressure required to penetrate a soil sample with a plunger of standard area. The measured pressure is then divided by the pressure required to achieve an equal penetration on a standard crushed rock material.

or

The test is performed by measuring the load required to penetrate a soil sample with a plunger of standard area. The measured load is then divided by the load required to achieve an equal penetration on a standard crushed rock material.

CALIFORNIA BEARING RATIO (CBR)

- (a) Developed by the California Division of Highways around 1930
- (b) Subsequently adapted by numerous states, counties and U.S. federal agencies. Adopted by the U.S. Army Corps of Engineers during early 1940's (WW II use).
- (c) This test is a <u>comparative</u> measure of the shearing resistance of a material and is used with empirically derived curves to design flexible pavement structures.
- (d) This test can be used for base. subbase. and subgrade materials.



California Bearing Ratio = (Load required for a certain penetration of plunger) x 100 Standard Load for same penetration • Surcharge plates are used on the mould to simulate the field conditions One surcharge plate represents 2.5" thick pavement Two surcharge plates represent 5" thick pavement (f) Apply load to piston at a rate of 1.3 mm (0.05") per minute. Record total load readings at penetrations ranging from 0.64 mm (0.025 in.) up to 7.62 mm (0.300 in.)

CBR(%) = (100) (x/y)

where x = material resistance or the unit load on the piston (pressure) for 2.54 mm (0.1") or 5.08 mm (0.2") of penetration

y = standard unit load (pressure) for well graded crushed stone

= for 2.54 mm (0.1") penetration = 6.9 MPa (1000 psi)

= for 5.08 mm (0.2") penetration = 10.3 MPa (1500 psi)

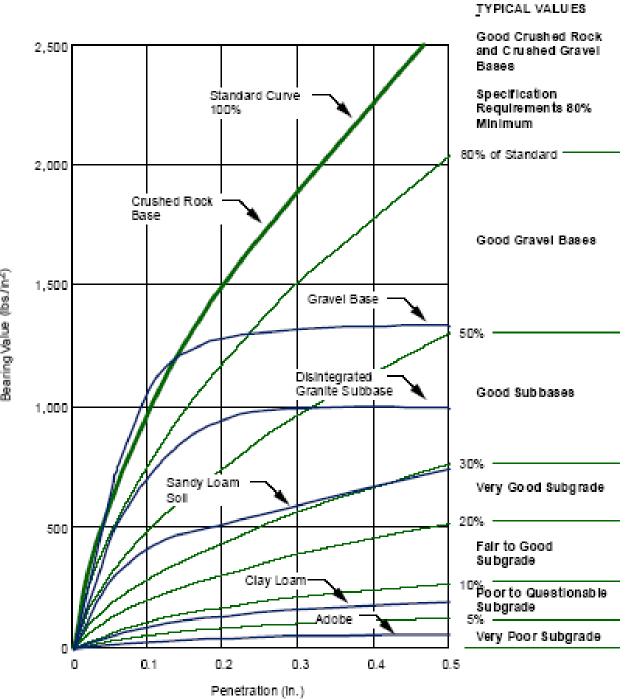
<u>Thus</u> CBR compares the material being tested with the bearing of a <u>well-graded</u> <u>crushed stone</u>. Thus, a high quality crushed stone base material should have a CBR \simeq 100%.

Standard Test Methods

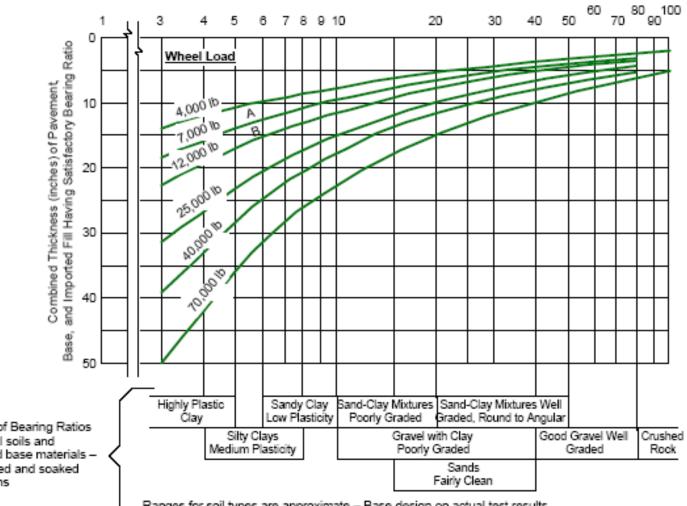
- AASHTO T193: The California Bearing Ratio
- ASTM D1883: Bearing Ratio of Laboratory Compacted Soils

$$CBR_{0.1"} = \underline{Load required for 0.1" penetration of plunger (lb.)}{3000}$$
$$CBR_{0.2"} = \underline{Load required for 0.2" penetration of plunger (lb.)}{4500}$$

•Higher of the two values will be used for design •Usually $CBR_{0.1"} > CBR_{0.2"}$



Bearing Value (Ibs/In²)

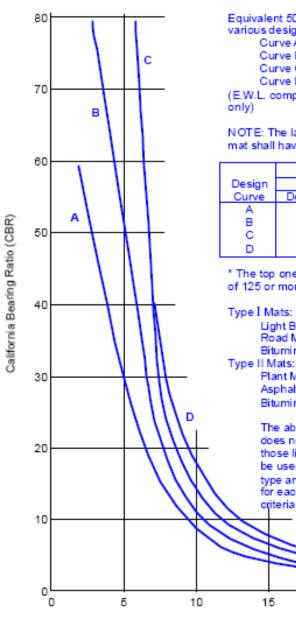


California Bearing Ratio in Percent at 0.1 Inch Penetration for Compacted and Soaked Specimen

Ranges for soil types are approximate - Base design on actual test results.

Ranges of Bearing Ratios for typical soils and untreated base materials compacted and soaked specimens

SURFACING DESIGN CURVES



Equivalent 5000 lb, wheel load repetitions (E.W.L.) for various design curves

| Curve A: | 0 | to | 200,000 |
|----------|---------|--------|-----------|
| Curve B: | 200,000 | to | 600,000 |
| Curve C: | 600,000 | to | 2,000,000 |
| Curve D: | over 2 | 2,000, | 000 |

(E.W.L. computed for 10 years traffic in one direction only)

NOTE: The layer immediately under the bituminous mat shall have the following minimum depth & CBR:

| | Bituminous Mat Type | | | |
|--------|---------------------|-----|-------|-----|
| Design | | I | I | I |
| Curve | Depth | CBR | Depth | CBR |
| A | 1" | 125 | | |
| в | 2" | 125 | 2" | 80* |
| с | 5" | 125 | 3" | 80* |
| D | 5" | 125 | 3" | 80* |

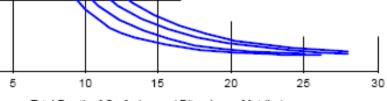
* The top one inch of this layer shall have a CBR value of 125 or more.

Type I Mats:

Light Bituminous Surface Treatment Road Mix Bituminous Macadam less than 2" thick

Plant Mix 2" and thicker Asphaltic Concrete 2" and thicker Bituminous Macadam 2" and thicker

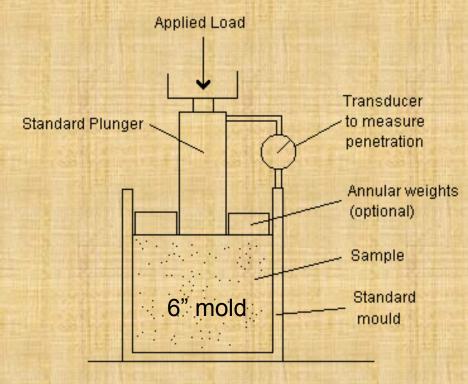
The above classification of bituminous mats does not imply equivalence in service between those listed under a given type. The table is to be used for the design of the base only. The type and thickness of bituminous mat suitable for each project is to be determined from other criteria.



Total Depth of Surfacing and Bituminous Mat (in.)



Pavement Design California Bearing Ratio (CBR) *"The Test"*



Penetrations of 0.05" per minute

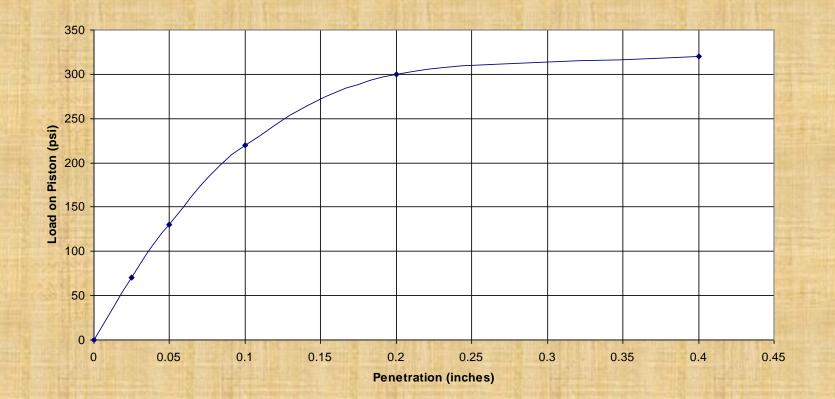
Take load readings at different penetrations

| 0.025" | 70 psi |
|--------|---------|
| 0.05" | 130 psi |
| 0.1" | 220 psi |
| 0.2" | 300 psi |
| 0.4" | 320 psi |

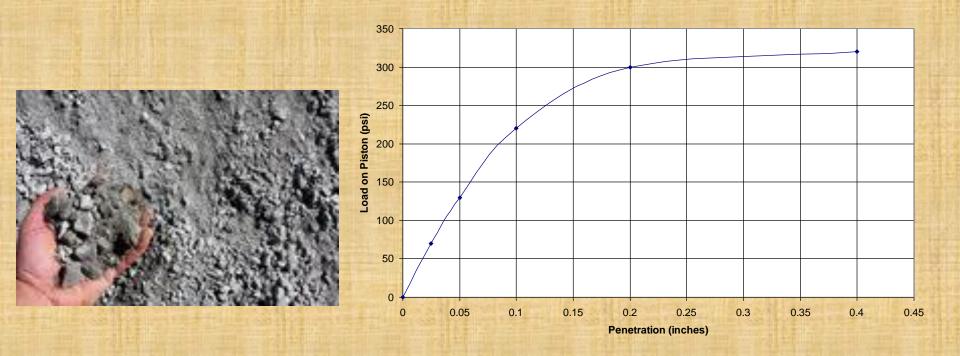


Achieve OMC & Max. Dry Unit Wt.

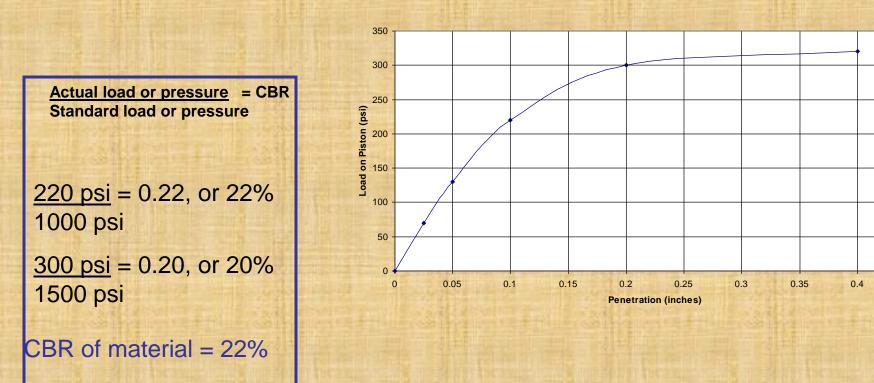
Plot the Data:



Determine the CBR values for the 0.1" and 0.2" penetration.



"The Gold Standard" for CBR Example above: for 0.1" of penetration, 1000 psi (3000 lb.) for 0.1" of penetration, 220 psi for 0.2" of penetration, 1500 psi (4500 lb.) for 0.2" of penetration, 300 psi The standard material for this test is crushed California limestone



"The Gold Standard" for CBR for 0.1" of penetration, 1000 psi for 0.2' of penetration, 1500 psi

Example above: for 0.1" of penetration, 220 psi for 0.2" of penetration, 300 psi 0.45

Use 0.1" of penetration, unless 0.2" is the greater value. •If so, then rerun the test, taking the higher of the two values from this second trial Actual load or pressure = CBR Standard load or pressure

<u>220 psi</u> = 0.22, or 22% 1000 psi

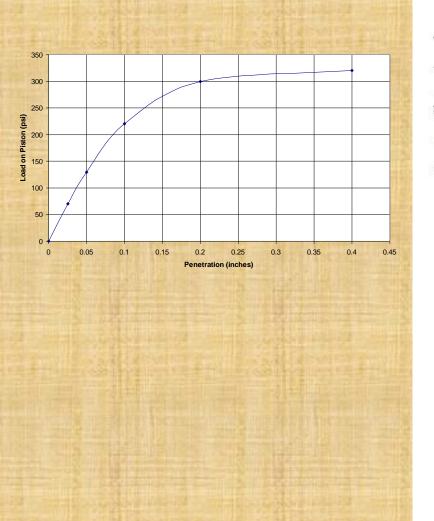
<u>300 psi</u> = 0.20, or 20% 1500 psi

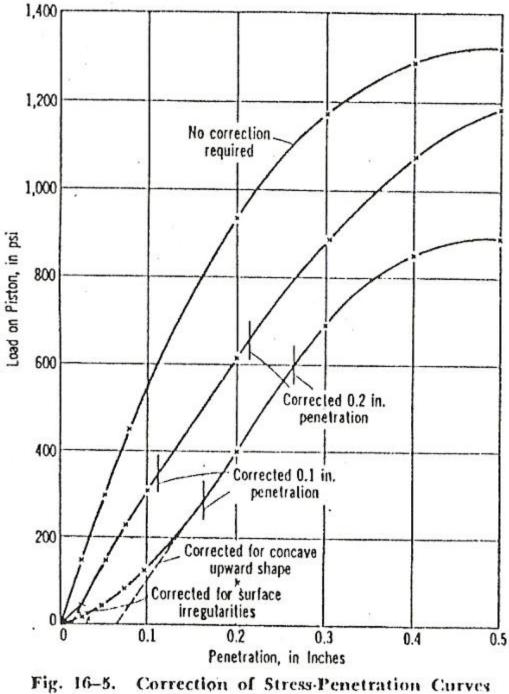
CBR of material = 22%, or "22" In General:

The harder the surface, the higher the CBR rating.
A CBR of 3 equates to tilled farmland,
A CBR of 4.75 equates to turf or moist clay,
Moist sand may have a CBR of 10.
High quality crushed rock has a CBR over 80.
The standard material for this test is crushed California limestone which has a value of 100.

"The Gold Standard" for CBR for 0.1" of penetration, 1000 psi for 0.2' of penetration, 1500 psi

Example above: for 0.1" of penetration, 220 psi for 0.2" of penetration, 300 psi Potential Corrections to the Stress-Penetration Curves



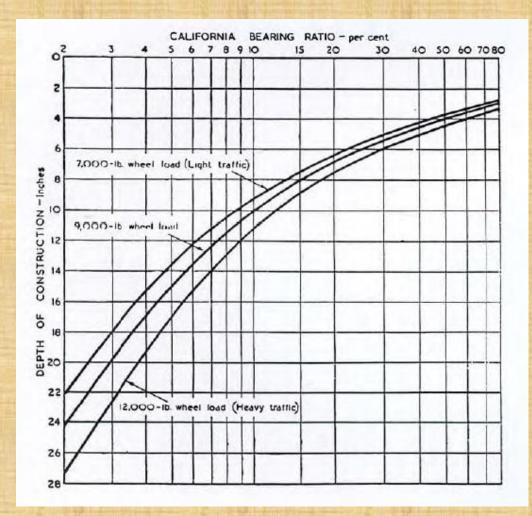


(Corps of Engineers)

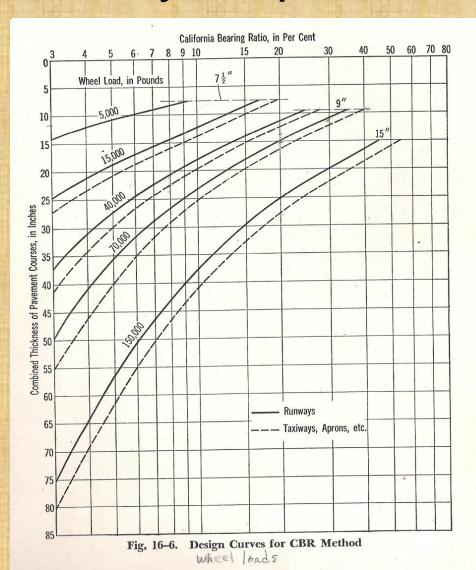
Pavement Design

- CBR values of the subgrade
- Type of use expected
- Expected wheel load during service
- Types of materials available for the construction

Design Curves for Roads

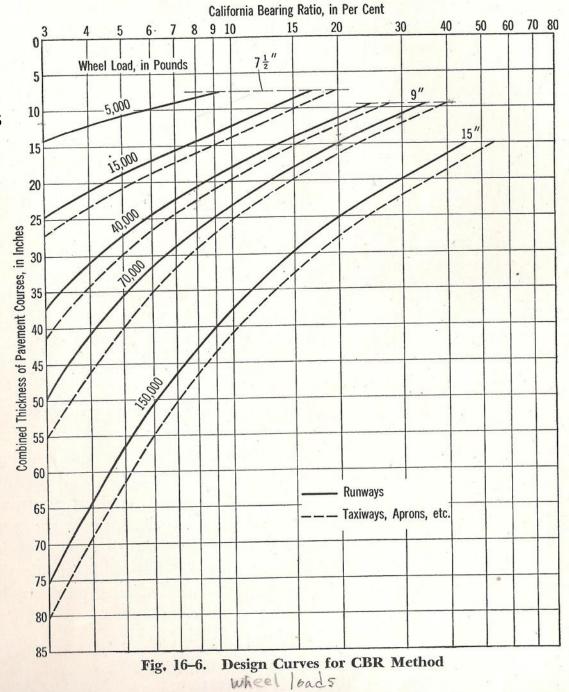


Design Curves for Runways, Taxiways, Aprons etc.



Example

A compacted subgrade has a CBR value of 8. What is the minimum pavement thickness if it is to support a taxiway pavement designed to support a 80,000 lb. airplane (40,000 lb. wheel load)? A point on the curve for a given CBR material represents the minimum thickness of pavement courses that will reside above it, in order to maintain stability



CBR of subgrade = 8 (Taxiway) Wheel load = 40,000 lb.

California Bearing Ratio, in Per Cent 50 60 15 30 40 9 10 20 9 0 71" Wheel Load, in Pounds ç 9" 5,000 10 15" 15 isian 20 23 inches aojar 25 in Inches 30 10:00 Combined Thickness of Pavement Courses, 35 40 45 (5°00) 50 55 60 Runways 65 - Taxiways, Aprons, etc. 70 75 4 1 80 85 Fig. 16-6. Design Curves for CBR Method whicel loads

Total thickness of construction = 23"

Pavement Design The Mechanics of the Design A compacted subgrade has a CBR value of 8. What is the minimum pavement thickness if it is to support a taxiway pavement designed to support a 80,000 lb. airplane (40,000 lb. wheel load)? 23 inches What is the optimal wearing surface thickness? ^{3 inches} What is the optimal CBR value of upper 6 inches of base? 6 inches of CBR 65/80

| Wheel Loads | CBR Value |
|-------------|-----------|
| 15k or less | 50 |
| >15k - 40k | 65 |
| >40k - 70k | 80 |
| >70k -150k | 80+ |
| | |

Wearing Surface 0 -15k..... 2" >15k - 40k.....3" >40k - 55k.....4" >55k - 70k.....5" >70k.....6"

Typical Values of CBR

| Material | CBR | Elastic Modulus (psi) | |
|---------------------------------|----------|------------------------------|--|
| Crushed Stone (GW, GP, GM) | 20 - 100 | 20,000 - 40,000 | |
| Sandy Soils (SW, SP, SM, SC) | 5 - 40 | 7,000 - 30,000 | |
| Silty Soils (ML, MH) | 3 - 15 | 5,000 - 20,000 | |
| Clayey Soils (CL, CH) | 3 - 10 | 5 <mark>,000</mark> - 15,000 | |
| Organic Soils (OH, OL, PT) | 1 - 5 | < 5,000 | |

Source: WSDOT Pavement Guide Interactive CD-ROM

Typical Values of CBR

| General Soil Type | USC Soil Type | CBR Range | |
|----------------------|---------------|-----------|--|
| | GW | 40 - 80 | |
| | GP | 30 - 60 | |
| | GM | 20 - 60 | |
| Come minuterile | GC | 20 - 40 | |
| Coarse-grained soils | SW | 20 - 40 | |
| | SP | 10 - 40 | |
| | SM | 10 - 40 | |
| | SC | 5 - 20 | |
| | ML | ≤15 | |
| | CL | ≤15 | |
| Fine-grained soils | OL | ≤5 | |
| | MH | ≤10 | |
| | CH | ≤15 | |
| | OH | ≤5 | |

Source: WSDOT Pavement Guide Interactive CD-ROM

Rating of Materials using CBR

| California Bearing Ratio (%) | Rating |
|---------------------------------|-----------------------|
| 2 - 5 | Very Poor Subgrade |
| 5 - 8 | Poor Subgrade |
| 8 - 20 | Poor to Fair Subgrade |
| 20 - 30 | Excellent Subgrade |
| 30 - 60 | Good Sub-base |
| 60 - 80 | Good Base |
| 80 - 100 | Best Base |

Rating of Materials using CBR in Pakistan

| California Bearing | General Rating | Uses | Classification System | |
|-----------------------|-------------------|-------------------|---------------------------|-------------------------|
| Ratio (%) | | | USCS | AASHTO |
| 0 – 3 | Very Poor | Subgrade | OH, CH, MH,OL | A5, A6, A7 |
| 3 – 7 | Poor to Fair | Subgrade | OH, CH,MH, OL | A4, A5, A6, A7 |
| 7 – 20 | Fair | Sub-base | OL, CL, ML, SC, SM, SP | A2, A4,A6, A7 |
| 20 – 50 | Good | Base, Sub-base | GM, GC, SW, SM, SP, GP | A1-b, A2-5, A3, A2-6 |
| > 50 | Excellent | Base | GW, GM | A1-a, A2-4, A3 |